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CONSULTING

File No.:VA201-141/4-A.01
Cont. No.:VA11-00817

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August 22, 2011

Mr. Ralph Shearing, President
Soho Resources Corp.
250 - 1090 West Georgia Street
Vancouver, BC V6E 3V7

Dear Ralph,

Re: Tahuehueto Project – Initial Site Visit and Preliminary Site Assessment

1.0 INTRODUCTION

Soho Resources Corp. (Soho) has retained Knight Piésold Ltd. (KP) to provide pre-feasibility level geotechnical design recommendations for the Tahuehueto Project. The Tahuehueto Project is located in Durango State, Mexico as shown on Figure 1. Soho is currently at an advanced exploration stage of developing this structurally controlled epithermal gold-silver and associated copper-lead-zinc deposit, which includes three mineable resources – El Creston, El Rey and Cinco de Mayo. The pre-feasibility study will further develop the mining plans and concepts presented in the Preliminary Economic Assessment (PEA) completed by Snowden Mining Industry Consultants Inc. (Snowden, October 2010).

The geotechnical design work has been separated into two phases. The initial phase (Phase 1) targets high priority items identified in the PEA, which include the geotechnical assessment of the El Creston pit, the potential development of a Cinco de Mayo open pit, and the general layout of the waste and water management facilities. The second phase (Phase 2) is to complete site investigations and engineering studies relating to waste and water management and environmental management for a 43-101 compliant pre-feasibility study. The scope of work for the Phase 1 study includes:

- A site visit to collect geotechnical and geomechanical data.
- Geomechanical data characterization and slope stability assessment to develop preliminary recommendations for open pit slope design.
- Sizing and layout development of waste management facilities described in the PEA based on the site visit geotechnical findings.
- Integration of mine components to develop a recommended base case site layout for the pre-feasibility study.
- Identification of opportunities for improvement in the mine development concept for the pre-feasibility studies, which includes the potential development of an open pit at the Cinco de Mayo deposit and alternate waste rock dump layouts.

The proposed general arrangement of the Tahuehueto Project is illustrated on Figure 2 and an isometric overview of the project site is shown on Figure 3. This letter summarizes the findings and recommendations of the Phase 1 preliminary study.

2.0 SITE VISIT SUMMARY

A site visit to the Tahuehueto Project was completed by Mr. Greg Johnston, P.Eng., and Ms. Louise Shannon, P.Eng., of Knight Piésold Ltd. from April 26 to May 2, 2011. During this site visit, the following activities were completed:

BSI
ISO 9001, ISO 14001
OHSAS 18001

- Geotechnical mapping within select areas of the existing historic underground workings. The historic underground workings provide good access to portions of the ore bodies: El Creston, El Rey, and Cinco de Mayo.
- A 'Proof of Concept' review of the SiroVision™ 3D imaging analysis method for use in data collection of the cliffs above the El Creston and Cinco de Mayo deposits.
- A general reconnaissance for potential plant site locations.
- A reconnaissance and geotechnical review of the waste management facility locations including the waste rock dump and tailings storage facility.
- Assistance with implementation of the hydrology data collection program.

The KP site visit was assisted by Soho site manager and geologist, Ing. Ricardo Cruz Gatica, providing logistical and technical support, project access and summarized the geological understanding of the site. A number of locations on the project site were visited and selected site photos are presented in Appendix A. The locations visited for geotechnical mapping and data collection are shown on Figure 4 and summarized as follows:

- The core yard and old mill site, see Photos 1 and 2.
- El Creston Underground Level 16, see Photo 3.
- El Creston Underground Level 14, see Photo 4.
- El Creston Underground Level 10, see Photo 5, 6 & 7.
- El Rey Underground Level 4, see Photo 8.
- El Rey Underground Level 2 (limited access), see Photo 9.
- Cinco de Mayo Underground Level 1, see Photo 10.
- The toe and crest of the rock cliff exposures above the El Creston deposit, see Photo 11 and 12.
- The crest of the high rock cliffs above the Cinco de Mayo deposit, see Photo 13.
- Select sites within the waste rock dump footprint, see Photo 14.
- The weather station and hydrology weirs, see Photo 15 and 16.
- The portion of the site access road from the plant site to the river Rio de las Vueltas, see Photo 17.

Geotechnical Mapping

A geotechnical mapping program was conducted within select areas of the existing underground workings. The geotechnical mapping locations were selected based on accessibility, location within the proposed mine development and the orientation of the underground workings (to observe the rock mass at various orientations). The geotechnical mapping was completed at the following locations:

- El Creston: Levels 10, 14, and 16.
- El Rey: Level 2 (limited to the portal entrance) and Level 4.
- Cinco de Mayo: Level 1.

The existing underground workings were generally found to be in good condition, and the underground levels mapped represent a limited selection of the accessible underground levels.

The geotechnical mapping included data collection on the overall Rock Mass Rating (RMR), and individual discontinuities. Discontinuity measurements included structural orientations, joint condition parameters such as infill material, joint roughness coefficient (JRC), as well as water flow characteristics. An estimation of unconfined compressive strength (UCS) was made for the intact rock at each mapping station.

The mapping data was utilized for a preliminary pit slope geotechnical assessment, which will be discussed in Section 3.0.

SiroVision 'Proof of Concept'

Structural surface mapping of the upper cliffs in both the Cinco de Mayo and El Creston areas was not feasible due to the height and near vertical nature of the cliffs (see Photos 11, 12, 13 and 14). The SiroVision photogrammetry method was trialed as a 'proof of concept' to collect structural mapping data from the cliffs. SiroVision uses pairs of high resolution digital photos in conjunction with photogrammetry software to create digital 3D models of rock mass structures for geotechnical analysis. The benefits of using SiroVision include:

- Ability to map faces and structures that are difficult to access or hazardous to map conventionally.
- High levels of spatial accuracy when calibrated correctly.

SiroVision photographs pairs were taken for the Cinco de Mayo and El Creston cliffs during the site visit. The photographs were completed by KP staff and analyzed by TerraSource Software Ltd. (TerraSource). This initial SiroVision program was scoped to confirm the system is capable of collecting data required for the analysis of the high cliffs under the site conditions. The cliffs of interest are located above the El Creston and Cinco de Mayo deposits and the associated proposed pits.

A first set of SiroVision images were captured the morning of April 28th of the cliffs in the El Creston area. The images were sent to TerraSource and adjustments were made to the program setup based on the preliminary analysis. A second set of photographs of the El Creston area cliffs and the Cinco de Mayo cliffs were collected on the 30th April and 1st of May. Based on preliminary analysis results, the SiroVision system is expected to be suitable for future data collection at the Tahuehueto site.

Hydrology Weirs

A series of six hydrology weirs have been constructed in the valleys and gullies (arroyos) of the Tahuehueto project site. The weir locations are based on the PEA recommendations. The purpose of the weirs is to facilitate the measurement of surface water flow to improve the understanding of the hydrology in the project area. Measurement of steam flows in areas with steep slopes and short intense rainfall events that occur in the project area is difficult. The weirs will improve the accuracy of stream flow measurement by directing the water flow through a constructed outlet notch, the stream flow rate can be calculated based on the outlet notch size and water level behind the weir. Stream flow rates in the area are expected to be high during storm events and there have been debris flows recorded in past rainy seasons.

Field inspections to all of the weir locations were conducted and modifications to several of the existing structures were discussed on site to improve the performance and accuracy of the weirs. The weirs may be improved by:

- A minor modification to the existing Arroyo Cinco de Mayo weir to ensure flow does not escape at the abutments (see Figure 4).
- Change the weir exit from the current rectangular notch to a V-notch. It would also be beneficial to put a small metal edge on the downstream face of the weir. This can be accomplished by bolting a thin strip of metal to the concrete. For V-notch weirs, a carpenter's square can be used, the thin edge helps to control the water passing through the weir so that the flow can be more accurately calculated.
- The installation of stream gauge sensors and data loggers in the upstream stilling pond to provide a continuous record of the steam flow. The stream gauge sensors should be installed with protection from debris. The PT2X sensor/datalogger package has been suggested as appropriate for the project site and Ing. Ricardo Cruz Gatica has been put in touch with a Latin American supplier of the sensors.

3.0 PRELIMINARY OPEN PIT SLOPE ASSESSMENT

The stability of large open pit rock slopes is typically controlled by geology, structure, rock mass characteristics, groundwater conditions and operating factors such as blasting practices and mining sequence. Typically open pit slope configurations include catch benches, inter-ramp slope angles and overall slope angles. The Figure 5 illustrates the relationship between bench geometry, inter-ramp slope angle and overall slope angle.

Deposit Geology

The Tahuehueto project site lies within the Sierra Madre Occidental mountain range in the state of Durango, Mexico. Rugged, mountainous terrain and steep topography dominate the site. The geology is described in the PEA (Snowden, 2010) as:

The Sierra Madre Occidental is a volcanic mountain range that consists of a lower volcanic series of late Cretaceous-Paleocene andesitic volcanic rocks and an upper volcanic series of Eocene to Oligocene silicic ignimbrites. An unconformity separates the lower and upper series (Wilson, 2009).

The property contains four main rock types: lower volcanic series andesite, granodiorite stocks, polymictic conglomerate, and felsic ash-flow tuffs of the upper volcanic series. The majority of the project area is underlain by andesite flows, tuffs, and volcaniclastic rocks of the lower volcanic series. The lower volcanic series remains generally undifferentiated. A volcaniclastic unit distinct from the andesite flows exists in the Texcalama and Cinco de Mayo areas and an andesite lithic lapilli tuff exists in the footwall of the El Creston structural zone. Granodioritic stocks intrude the andesites and are exposed at surface in the footwall of the El Creston structural zone and the El Rey mine area. The andesites and granodiorite are overlain by a basal polymictic conglomerate unit that is tens of meters thick and marks the unconformity between the lower and upper volcanic series. Amygdaloidal basalt flows occur locally within the conglomerate unit. In some areas, thin units of ignimbrite were deposited before the conglomerate. Late Tertiary or Quaternary landslides obscure outcrop patterns in the El Creston-El Perdido area and are likely to be present in other areas of steep topography within the project area.

Structural Geology

The general structural features at the Tahuehueto site are summarized from the PEA report (Snowden, 2010) as below:

A series of northeast-striking veins that formed within a series of normal faults with subordinate left-lateral displacement hosts the mineral resources. The principal, through-going veins have a general strike of 045° to 060° and dip between 65° and 80° to the southeast. This vein set includes Cinco de Mayo, El Catorce, and El Perdido and extends north-eastward to Santiago. Other veins with the same orientation include El Rey, Dolores, Tahuehueto, Texcalama, El Espinal, and Tres de Mayo. Within the core area of the mineral resources, the El Creston series of veins, striking about 035° and dipping 60° to 80° east, formed in a strongly dilatant zone between the through-going El Perdido and El Rey structures (Wilson, 2009).

Rock Mass Structure

The historic underground workings in the El Creston, Cinco de Mayo, and El Rey areas follow fault breccia zones associated with mineralization. Several smaller faults are encountered cross cutting the drifts. These small faults often have clayey infill and are in the order of 5 to 15 cm wide. Occasional wider fault zones were encountered in Level 10 of the El Creston underground work. Veins and other structures have been mapped by Soho site geologists, but this data has not yet been incorporated into the database for this initial study.

The majority of the joints and fault were found to be steeply dipping, planar with gentle undulations. Some small wedge failures were noted in the ceiling and walls of the underground workings. Only limited structural orientation data was collected during the site visit. Figures 6 to 8 present the stereographic plots of measured structural features at the El Creston, El Rey and Cinco de Mayo underground workings, respectively. The measured rock mass structural orientations are generally consistent with the northeast or northwest striking faulting and veining features, particularly at the Cinco de Mayo and El Rey deposits. However, mapping data at El Creston shows several predominant steeply dipping joint sets that strike perpendicular to the regional faulting and veining structures (see Figure 6).

Rock Mass Quality

A general site observation is that the intact rock is generally strong to very strong, and typically slightly weathered to fresh, and blocky. The overall rock mass is generally competent with weaker areas seen in zones of fault breccia. Occasional failure zones were noted in the roof (back) of the underground drifts that were generally associated with fault breccia zones. Fault breccia zones were slightly to moderately weathered with tight to open joints or voids. The persistence of joint or fault features was occasionally observed to be longer than 20 m, however, the majority of features had a persistence in the order 1 to 3 m.

The majority of the underground workings were dry with very limited groundwater inflow observed. In deeper portions of the drifts limited groundwater inflows were encountered, groundwater inflows were typically seen as steady dripping or dispersed water seeping along joint surfaces, which were encountered most noticeably in Level 16 of El Creston

An average rock mass rating (Bieniawski, 1989) value was estimated for the andesite rock by transposing the field measurements into the rock mass rating (RMR) parameters for rock mass classification. The RMR values were averaged via the arithmetic mean for each station, those station values were then averaged to obtain average recorded values for the overall mapped sections of the andesite. These field values were then matched with a corresponding RMR value, and summed based on the RMR methodology. This calculation resulted in an average RMR value of 66, classifying the andesite as a Rock Mass Class II - GOOD rock. This is likely a conservative estimate due to the inclusion of anomalies such as isolated very wet or weak discontinuities that are included in the averaging process.

Hydrogeology

The project is located in a region of moderate climate with wet and dry seasons. Temperatures are relatively high year round and precipitation is limited to defined parts of the year. The hydrogeology of the Tahuehueto site is observed to be complex with perched groundwater at higher elevations where at lower elevations in the same unit is dry.

Groundwater is flowing from some blast holes in the underground workings in Level 16 of El Creston and this water is being used by nearby residents. Several underground workings at El Rey are inundated with water due to small dams and blockages at the portals and are also possibly being utilized by local residents. There was evidence of higher water lines in some of the underground workings and the site manager described that the water could be pumped out of the drifts and would refill in a couple of days once pumping stopped. The Tahuehueto camp is using an exploration drillhole as a water source, which is located near the weather station just above the camp. Artesian conditions are observed during the rainy season.

El Creston

An open pit for the El Creston deposit was developed by Snowden for the PEA. The proposed El Creston open pit incorporates development of a 600-m high pit wall along the north side of the deposit in steep terrain. A 45-degree overall slope angle was used in the PEA pit design, and was based on Snowden's experience at other mine operations.

KP has reviewed the proposed pit design, the site topography and the recently collected structural mapping data. The exiting topography in the proposed El Creston pit includes natural ground slopes of 30 to 35 degrees over more than 1,000 m in elevation change. The ridges adjacent to the proposed pit include natural slopes of 45 degrees over 1,000 m of elevation change. Analysis of the structural mapping data identified a potential toppling failure mode for the north-northeast wall of the proposed El Creston pit, due to the presence of the northwest striking, sub-vertical structures (see Figure 6).

The recommended pit slope parameters for the development of the El Creston pit include a single bench (less than 15 m in height) configuration to reduce the potential for bench scale toppling failure. A relatively steep bench face angle of 65 to 75 degrees is likely achievable given the nature of the GOOD quality rock mass. A inter-ramp slope angle of 45 degrees is recommended to minimize the potential for multiple bench toppling failure. In addition, to provide additional rockfall containment and cleanout access for the proposed 600 m high pit wall wider benches and/or stepouts and haulage ramps are recommended at one third and two thirds of the ultimate pit on the north-northeast highwall. The resulting overall slope angle of the north-northeast wall will likely be in the order of 40 degrees. This recommended overall slope angle for the El Creston open pit is plotted on Figure 11 in comparison with published pit slope angles for hard rock mines. It shows that the recommended overall pit slope angle of 40 degrees is consistent with precedent at other mine sites.

The development of the El Creston open pit will require low damage controlled blasting and careful slope deformation monitoring throughout the entire life of pit operations.

Cinco de Mayo

The Cinco de Mayo deposit was intended to be mined using underground methods as described in the PEA. Soho has requested KP geotechnical input for the economic assessment of an open pit at the Cinco de Mayo deposit.

KP has reviewed recent mapping data together with the topography of the Cinco de Mayo deposit. It is anticipated that the potential Cinco de Mayo open pit will include a major highwall along the west side of the deposit. The existing natural ground slopes at around 40 degrees over an approximate height of 450 m. The measured structural features in this area are generally north striking and steeply dipping, which appear to be favorable for the west pit high wall excavation (see Figure 8). A preliminary rock mass slope stability model for the potential Cinco de Mayo open pit was developed to evaluate the global stability of the potential west pit wall. The rock mass strength parameters were based on the findings of the site observations and a back analysis of the existing slopes. The Cinco de Mayo open pit stability model incorporates a proposed offset from the crest of the pit to the base of the cliffs of 75 m. The material strength parameters and modeling results are presented in Figures 9 and 10. This preliminary analysis suggests it is reasonable to develop the West Wall of the open pit with an overall slope of 45 degrees. This recommended overall slope angle is comparable to published open pit mining precedent as shown on Figure 11.

The development of an open pit in the Cinco de Mayo area will require the exposed cliff face above the proposed pit be scaled to remove loose rock, and rock bolting of large features plus the development of rock fall catchment areas at the crest of the pit may be necessary to ensure worker safety.

4.0 WASTE MANAGEMENT FACILITIES

The mine will produce two main types of waste material to be managed on-site. These are waste rock primarily from the open pits and tailings from the processing of ore. The volume of waste rock and tailings are reported in the PEA (Snowden, 2010). The PEA includes the development of a waste rock dump (WRD) sized to store approximately 23 M tonnes (12 Mm^3) of material. This WRD is a cross valley type dump located within the catchment upstream of the Cinco de Mayo deposit.

In addition to the waste rock, approximately 9 M tonnes (6 Mm^3) of tailings will need to be managed in a safe, secure long term storage facility. Tailings samples are not currently available for testing for physical properties; the tailings have therefore been assumed to behave in a manner typical of epithermal gold-silver deposit tailings processed via conventional flotation.

Knight Piésold has completed a review of potential slurry tailings storage facility (TSF) sites in and around the project area. The project and surrounding area is characterized by very steep slopes and rugged terrain. Potential slurry TSF sites located within a reasonable distance of the deposit will have very large embankment volumes. The preliminary findings suggest that a slurry TSF would likely be impractical for the Tahuehueto Project. The management of tailings in a filtered dry stack type facility is considered the most practical at this time, these types of tailings management systems have been successfully implemented at other projects in mountainous areas of Mexico. It will be necessary to evaluate the particle size characteristics of the tailings solids; in general a coarse grind in the mill circuit will produce tailings that are more amenable to filtering for this dry stack option. The particle size distribution and rheology for the tailings will need to be reviewed to evaluate the potential for this tailings management methodology. Alternatively, it may possible to utilize highly thickened or paste tailings technology to develop a co-disposal strategy for the management of waste rock and tailings solids.

A combined tailings and waste rock dump has been developed that integrates the waste rock management and the tailings management as shown on Figures 2 and 3. The co-disposal facility has been sized to store of 35 M tonnes (20 Mm^3). This is in excess of the calculated total volume required and the additional volume provides expansion capacity. The combined tailings and WRD facility has final outer slopes of 26 degrees (2H:1V) to ensure stability during seismic events and to assist with reclamation of the dump surface at mine closure. A sediment control pond has been located downstream of the tailings/WRD facility to manage sediment and water quality.

The geochemical characteristics of both the waste rock and the tailings solids have not been considered in the current geotechnical assessment and will need to be evaluated further in the Phase 2 study program. Additional environmental control measures will be required if some of the waste materials are determined to be potentially acid generating or have adverse metal leaching characteristics.

5.0 MINE DEVELOPMENT CONCEPT

The PEA included layouts for the majority of the components of the mine (i.e. El Creston open pit, underground mine workings, waste rock dump and haul roads), and the conceptual locations of a plant site and tailings storage facility. KP has refined the conceptual PEA layout for the plant site and tailings management to produce a recommended base case mine general arrangement for the pre-feasibility study.

KP has evaluated potential locations for the development of a plant platform. The historic plant site is situated on steep terrain and it may not be feasible to develop and expand the existing site to accommodate a newer, larger plant. Potential sites for the new plant platform were evaluated based on topography and to be centrally located relative to the El Creston, El Rey and Cinco de Mayo deposits. The plant site shown on Figures 2 and 3 makes use of a relatively flat area to construct a plant site platform. This plant site location should be optimized in future studies.

The integration of the major mine facilities such as the open pit, waste rock dump, tailings storage and associated components is shown on Figures 2 and 3. This mine development concept is based on a combination of facilities described in the PEA and the findings of the KP site reconnaissance.

6.0 SUMMARY AND RECOMMENDATIONS FOR FUTURE WORK

The PEA geotechnical parameters used for the El Creston open pit were based on the Snowden experience with no geotechnical input. This preliminary review of site geotechnical conditions and a comparison with precedent practice in a large number of open pit mines confirmed that the geotechnical parameters used for the El Creston open pit are generally appropriate with an inter-ramp slope angle of 45 degrees and an overall slope angle of approximately 40 degrees. The next step in the pre-feasibility study (Phase 2) is to collect and analyze additional data to verify the preliminary study (Phase 1) results. The additional data will assist with the development of the pit slope design.

A mine development concept recommended for the pre-feasibility base case design has been developed based on the PEA and the findings of a geotechnical site review. During the development of this mine development concept, several additional areas that could improve the mine economics were identified as follows:

- The development or partial development of the Cinco de Mayo deposit by open pit methods as shown on Figure 2. Preliminary pit slope angles are described in Section 3.0.
- The development of a combined tailings/waste rock dump in the valley immediately below the El Creston pit. A preliminary storage capacity evaluation and layout has been completed as shown on Figure 2. This preliminary analysis shows sufficient capacity exists in the valley to store the combined waste rock and tailings, potential benefits of relocation are a reduced haul distance for waste, a reduced watershed catchment area to be managed, and the containment of major surface disturbances to within one valley.
- The development of a plant site and dry stack TSF on the relatively flat ground to the west of Cinco de Mayo deposit. The capital costs at this site for mill and initial TSF construction are expected to be lower. However, the operating costs may be higher due to the greater distance from the Cinco de Mayo and El Creston deposits. A desktop trade off study to evaluate the impact of capital and operating costs should be completed to evaluate the economic value of a mill site located further from the deposit.

The next steps in support of the pre-feasibility study are recommended as follows:

- The continued mapping of the historic underground workings by Soho personnel including orientations of structural features. This data should be entered into a database suitable for inclusion in the block model for ease of use and portability. The amount of work required to access additional levels in the historic underground workings that are currently flooded or otherwise inaccessible should be evaluated.
- Further evaluation and execution of SiroVision imaging survey for rock mass structural data collection.
- The continued collection of meteorology data from the site weather station. The instruments on the site weather station have passed their expiry date calibration, it was noted that the replacement of the

weather station was being planned. It is recommended that the weather station be replaced or the instruments be recalibrated.

- The establishment of stream flow data over the 2011 - 2012 wet season will improve the understanding of water resources available in the area. It is recommended that the weirs be inspected on a regular basis to ensure accurate data is collected and knowledge is developed on the performance of the weirs.
- Complete a preliminary geochemical assessment of waste rock and tailings materials and incorporate refinements to the mine plan to minimize adverse impacts to down gradient water quality.
- Evaluate water supply options for the proposed development.
- The completion of desktop based trade-off studies to evaluate the economic impact of relocating the plant site, waste rock dump and tailings storage facility.

We trust this letter meets your requirements in summarizing the findings of the Phase 1 study and providing recommendations for the next phase of work. Please contact the undersigned if you have any questions or comments.

Yours truly,
KNIGHT PIESOLD LTD.

PROFESSIONAL ENGINEER
PROVINCE OF BRITISH COLUMBIA
G. I. JOHNSTON
35676

Aug 22 2011

Signed:
Greg Johnston, P.Eng.
Senior Engineer

Reviewed:
Daniel Y. Yang, P.Eng.
Senior Geotechnical Engineer

Approved:
Ken J. Brouwer, P.Eng.
Managing Director

Attachments:

Figure 1 Rev 0	Project Location Plan
Figure 2 Rev 0	General Arrangement
Figure 3 Rev 0	General Arrangement – Isometric View
Figure 4 Rev 0	Underground Mapping Locations
Figure 5 Rev 0	Typical Pit Slope Configuration
Figure 6 Rev 0	Stereographic Analysis – El Creston Mapping Data
Figure 7 Rev 0	Stereographic Analysis – El Rey Mapping Data
Figure 8 Rev 0	Stereographic Analysis – Cinco de Mayo Mapping Data
Figure 9 Rev 0	Cinco de Mayo Rock Mass – Stability Analyses – Natural Slope Profile
Figure 10 Rev 0	Cinco de Mayo Rock Mass – Stability Analyses – Open Pit West Wall
Figure 11 Rev 0	Slope Height Versus Slope Angle Precedent for Hard Rock Slopes
Appendix A	Site Visit Photographs

Copy To: Jack Miller
Ricardo Cruz Gatica

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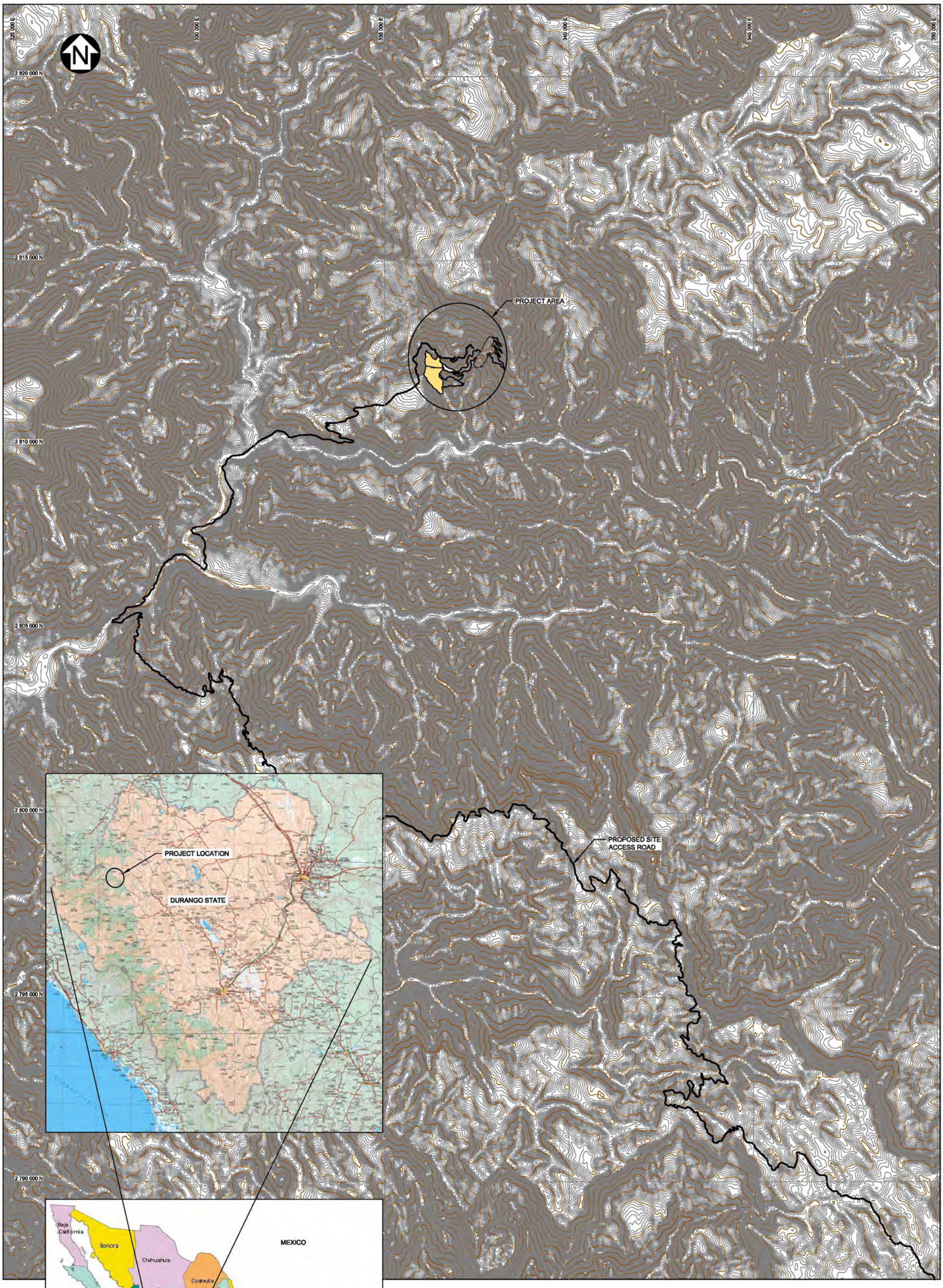

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SOHO RESOURCES CORP.

TAHUEHUETO PROJECT

PROJECT LOCATION PLAN

Knight Piésold CONSULTING

P/A NO. VA201-141/4

REF NO. VA11-00817

FIGURE 1

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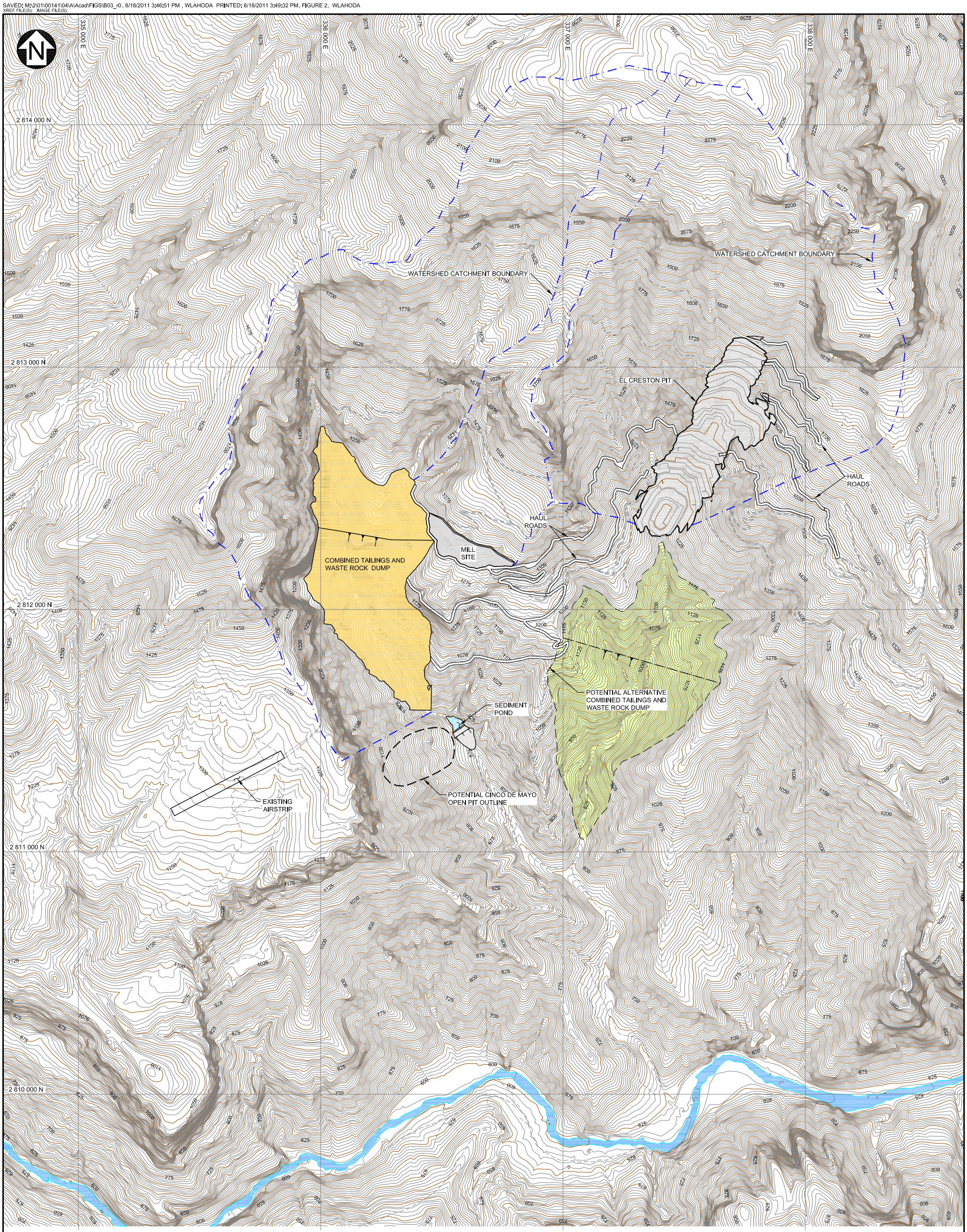

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NOTES:

1. PLAN/SECTION BASED ON INFORMATION PROVIDED BYSNOWDEN, DATED OCT' 2010.
2. CONTOUR INTERVAL IS 5.0 METRES.

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SOHO RESOURCES CORP.

TAHUEHUETO PROJECT

GENERAL ARRANGEMENT

Knight Piésold CONSULTING

P/A NO. VA201-141/4

REF NO. VA11-00817

FIGURE 2

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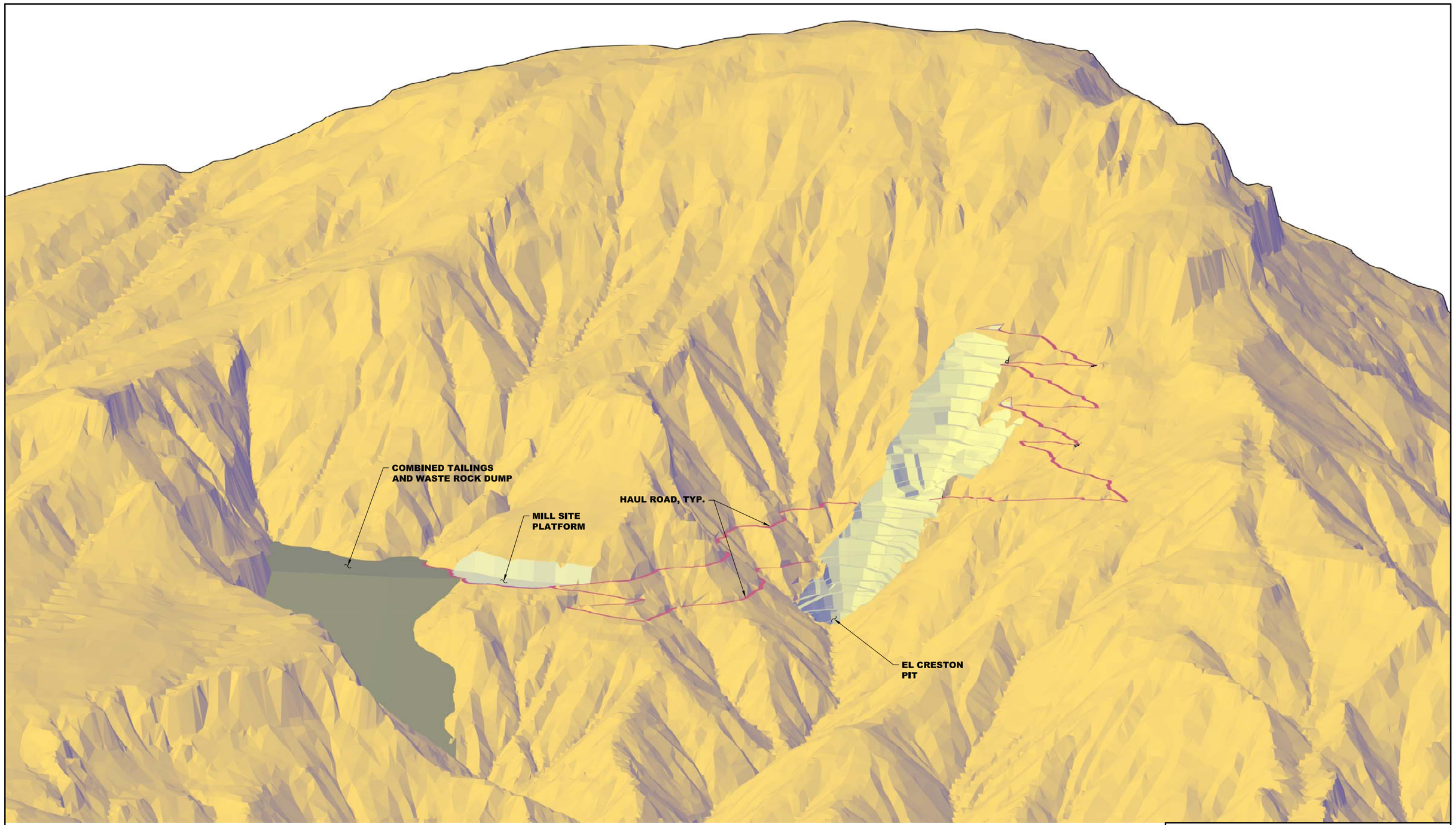

SOHO RESOURCES CORP.

TAHUEHUETO PROJECT

GENERAL ARRANGEMENT
ISOMETRIC VIEW

Knight Piésold CONSULTING

P/A NO.	REF NO.
VA201-141/4	VA11-00817

FIGURE 3 REV 0

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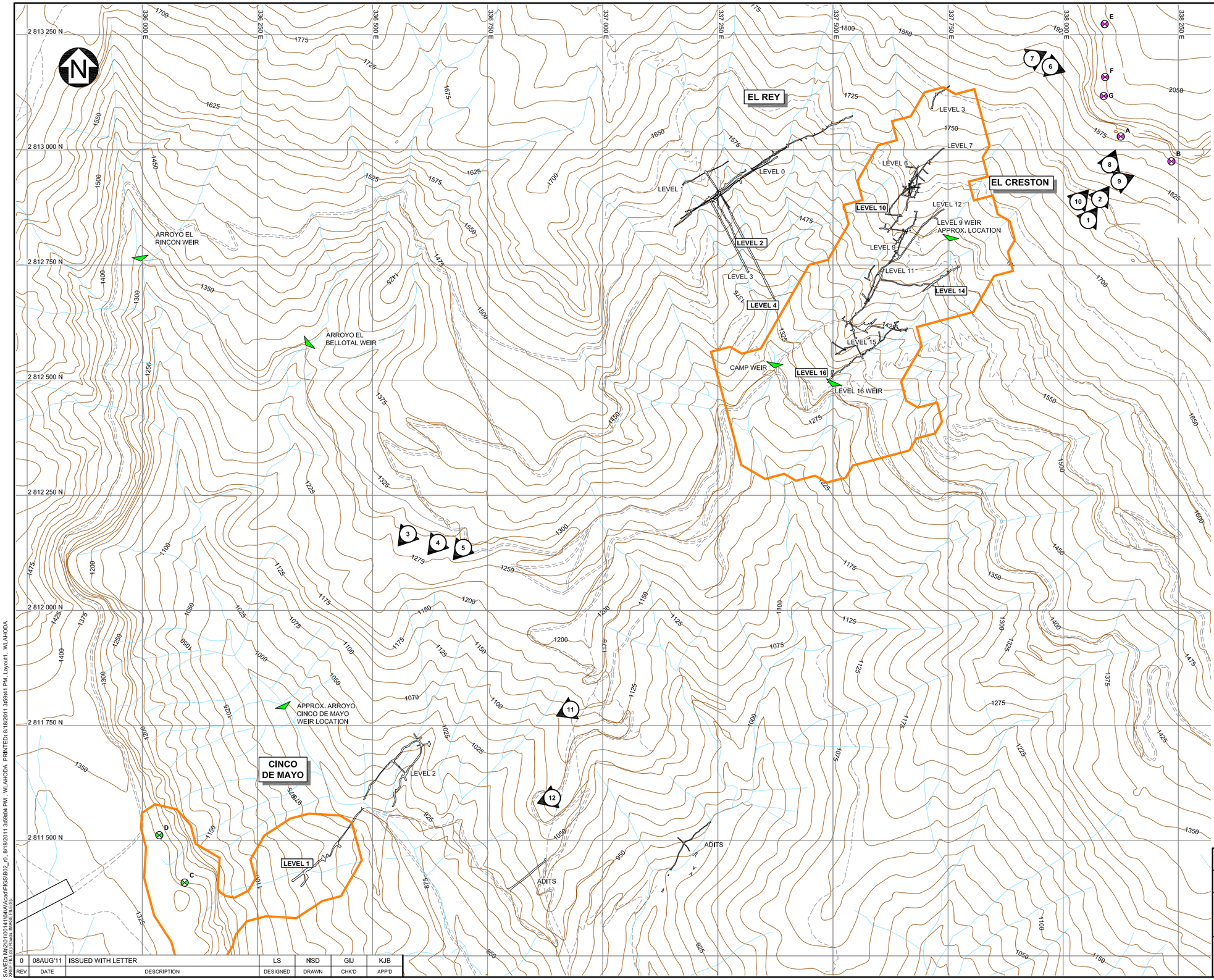

LEGEND:

- SIROVISION PHOTO LOCATION
- EL CRESTON SIROVISION MARKER
- CINCO DE MAYO SIROVISION MARKER
- WEIR LOCATION
- LEVEL 16 — HISTORIC UNDERGROUND WORKINGS WHERE MAPPING WAS COMPLETED
- LEVEL 16 — HISTORIC UNDERGROUND WORKINGS LEVEL
- DEPOSIT OUTLINE (PROVIDED BY SOHO RESOURCES CORP.)

NOTES:

1. COORDINATE GRID IS UTM WGS84.
2. PLAN BASED ON INFORMATION PROVIDED BY SOHO RESOURCES CORP, (MAY 01,2011).
3. CONTOUR INTERVAL IS 25 METRES.
4. DIMENSIONS AND ELEVATIONS ARE IN METRES, UNLESS NOTED OTHERWISE.

SCALE A 80 40 0 100 200 300 400 m

REV	DATE	DESCRIPTION	DESIGNED	DRAWN	CHK'D	APP'D
0	08AUG'11	ISSUED WITH LETTER	LS	NSD	GIJ	KJB

SOHO RESOURCES CORP.

TAHUEHUETO PROJECT

UNDERGROUND MAPPING LOCATIONS

Knight Piésold CONSULTING

P/A NO. VA201-141/4

REF NO. VA11-00817

FIGURE 4

REV 0

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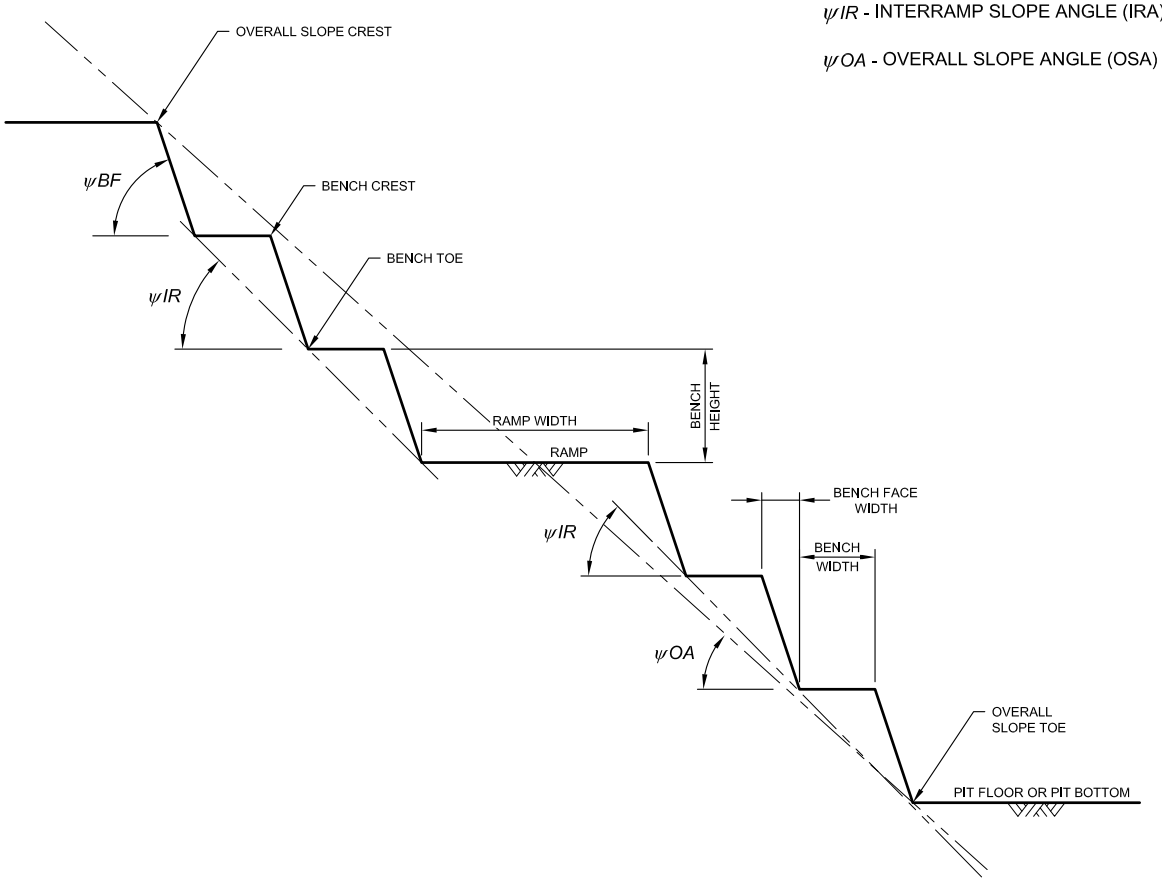
ψBF - BENCH FACE ANGLE (BFA)

ψIR - INTERRAMP SLOPE ANGLE (IRA)

ψOA - OVERALL SLOPE ANGLE (OSA)



REV	DATE	DESCRIPTION	DESIGNED	DRAWN	CHK'D	APP'D
0	08AUG'11	ISSUED WITH LETTER	GIJ	RP	DAY	KJB

SOHO RESOURCES CORP.

TAHUEHUETO PROJECT

TYPICAL PIT SLOPE CONFIGURATION

Knight Piésold CONSULTING

P/A NO. VA201-141/4

REF NO. VA11-00817

FIGURE 5

REV 0

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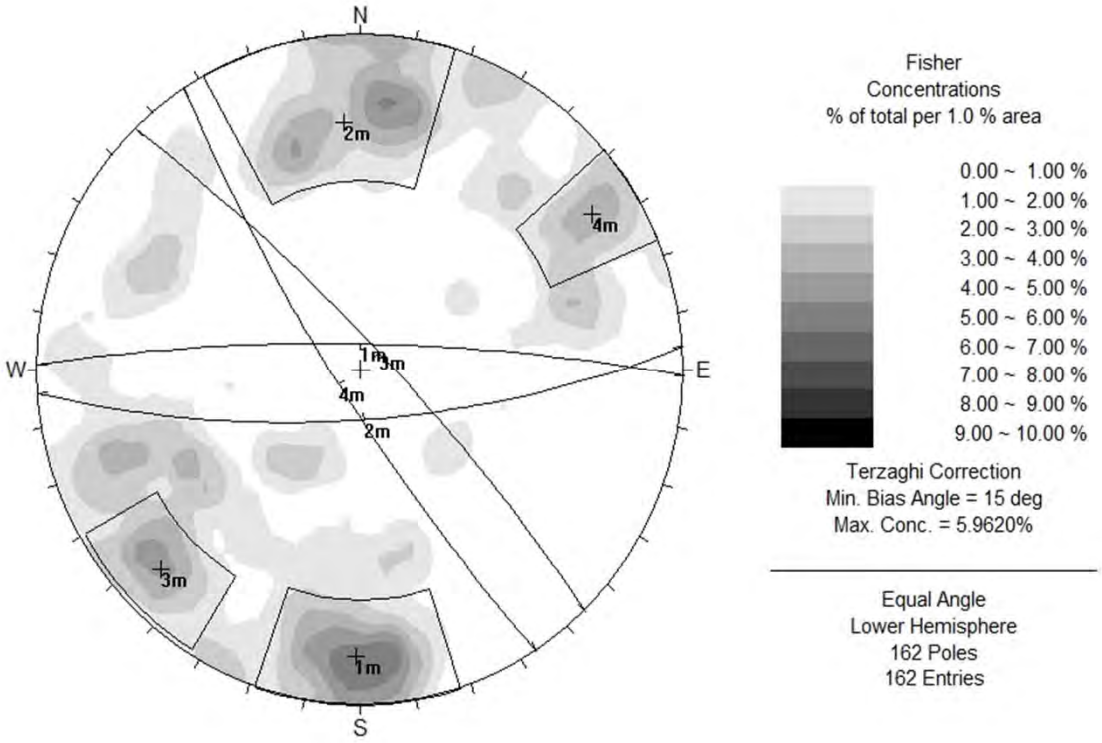


Location: El Creston - Levels 10, 14 and 16
Rock Type: Andesite

NOTE:
1. UNDERGROUND MAPPING DATA COLLECTED BY KP (APRIL - MAY 2011).

REV	DATE	DESCRIPTION	PREP'D	CHK'D	APP'D
0	08AUG'11	ISSUED WITH LETTER	LS	DAY	KJB

SOHO RESOURCES CORP.

TAHUEHUETO PROJECT

STEREOGRAPHIC ANALYSIS
EL CRESTON MAPPING DATA

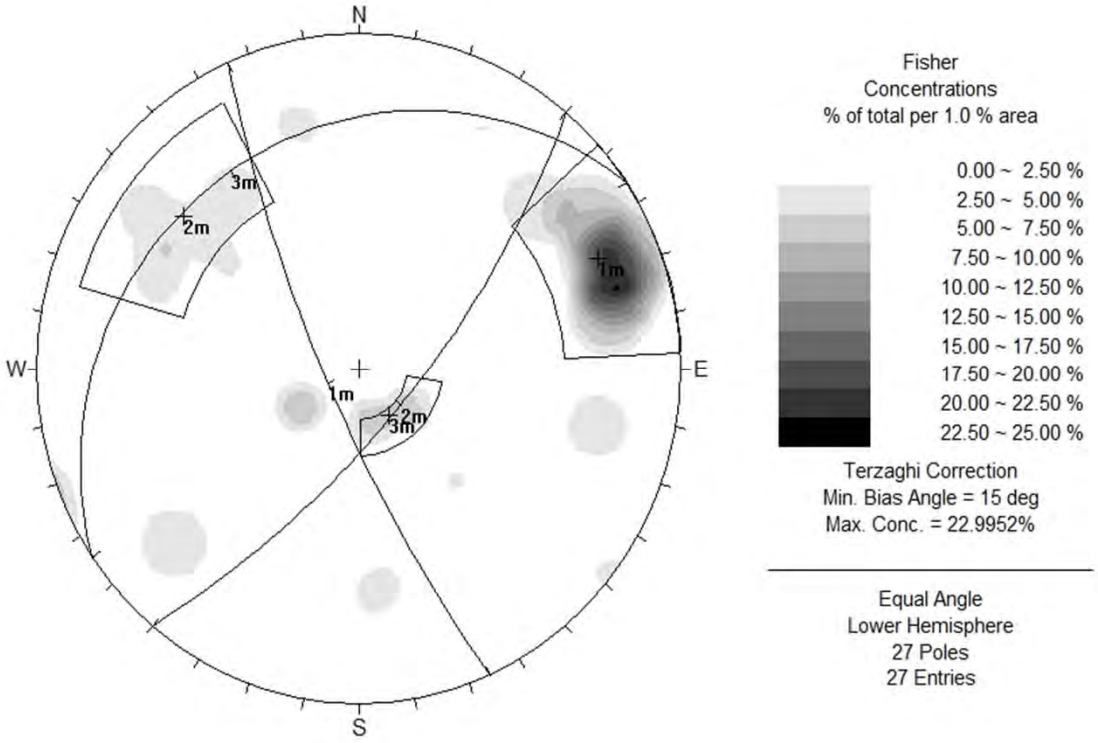
Knight Piésold CONSULTING

P/A NO. VA201-141/04

REF NO. VA11-00817

FIGURE 6

REV 0



Location: El Rey - Level 2 and Level 4
Rock Type: Andesite

NOTE:
1. UNDERGROUND MAPPING DATA COLLECTED BY KP (APRIL - MAY 2011).

REV	DATE	DESCRIPTION	PREP'D	CHK'D	APP'D
0	08AUG'11	ISSUED WITH LETTER	LS	DAY	KJB

SOHO RESOURCES CORP.

TAHUEHUETO PROJECT

STEREOGRAPHIC ANALYSIS
EL REY MAPPING DATA

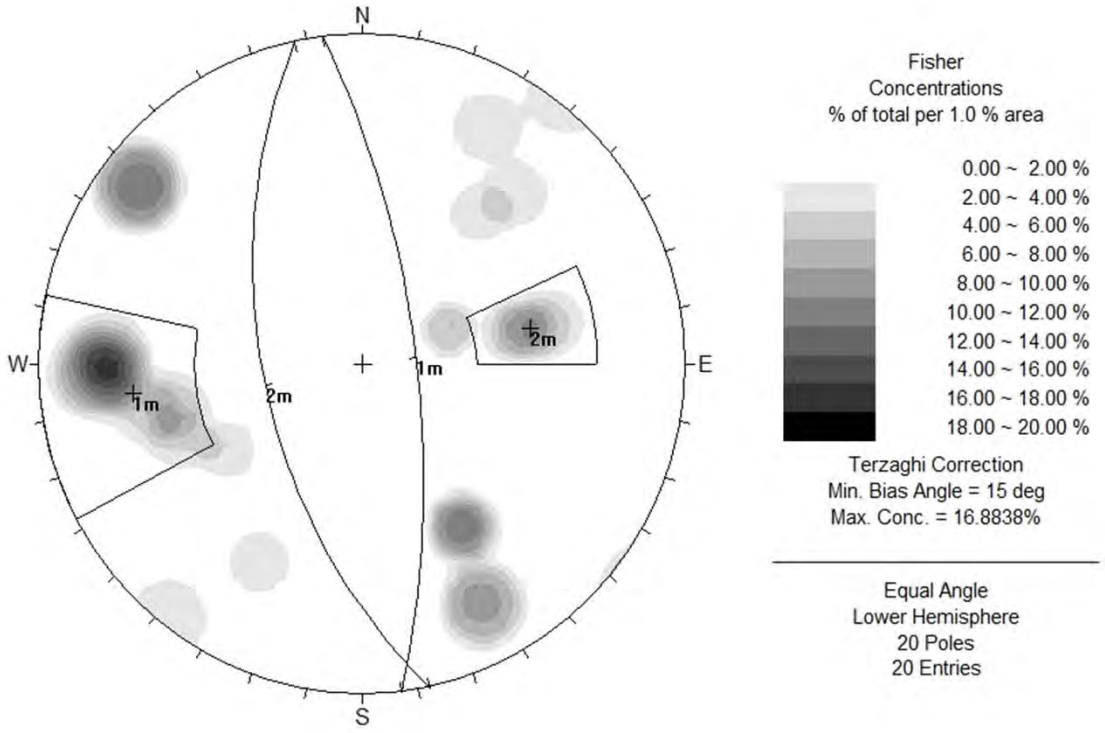
Knight Piésold CONSULTING

P/A NO. VA201-141/04

REF NO. VA11-00817

FIGURE 7

REV 0



Location: Cinco de Mayo - Level 1
Rock Type: Andesite

NOTE:
1. UNDERGROUND MAPPING DATA COLLECTED BY KP (APRIL - MAY 2011).

REV	DATE	DESCRIPTION	PREP'D	CHK'D	APP'D
0	08AUG'11	ISSUED WITH LETTER	LS	DAY	KJB

SOHO RESOURCES CORP.

TAHUEHUETO PROJECT

STEREOGRAPHIC ANALYSIS
CINCO DE MAYO MAPPING DATA

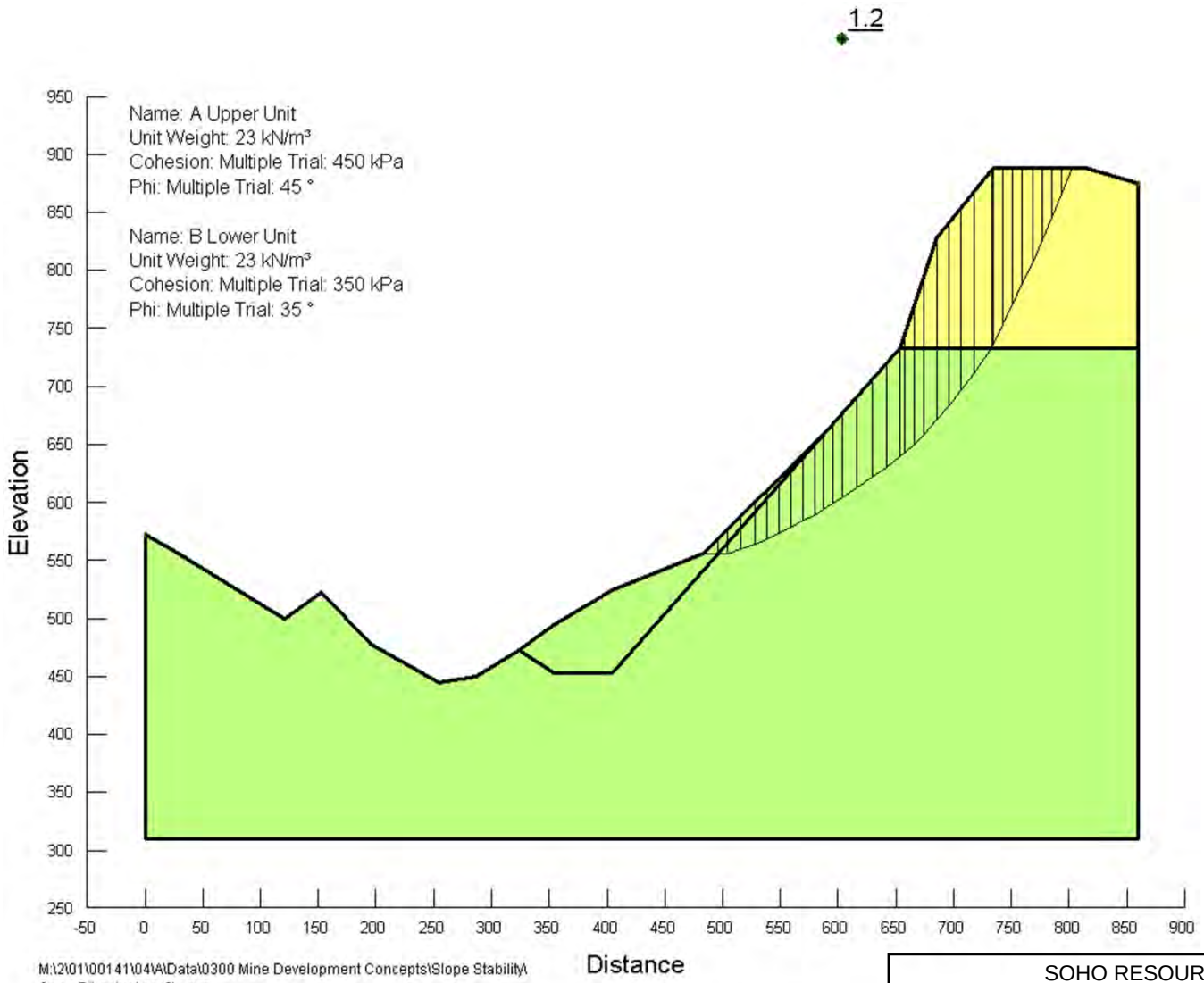
Knight Piésold CONSULTING

P/A NO. VA201-141/04

REF NO. VA11-00817

FIGURE 8

REV 0



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Name: SLOPE/W Analysis (FS)

REV	DATE	DESCRIPTION	PREP'D	CHK'D	APP'D
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SOHO RESOURCES CORP.

TAHUEHUETO PROJECT

CINCO DE MAYO
ROCKMASS STABILITY ANALYSES
NATURAL SLOPE PROFILE

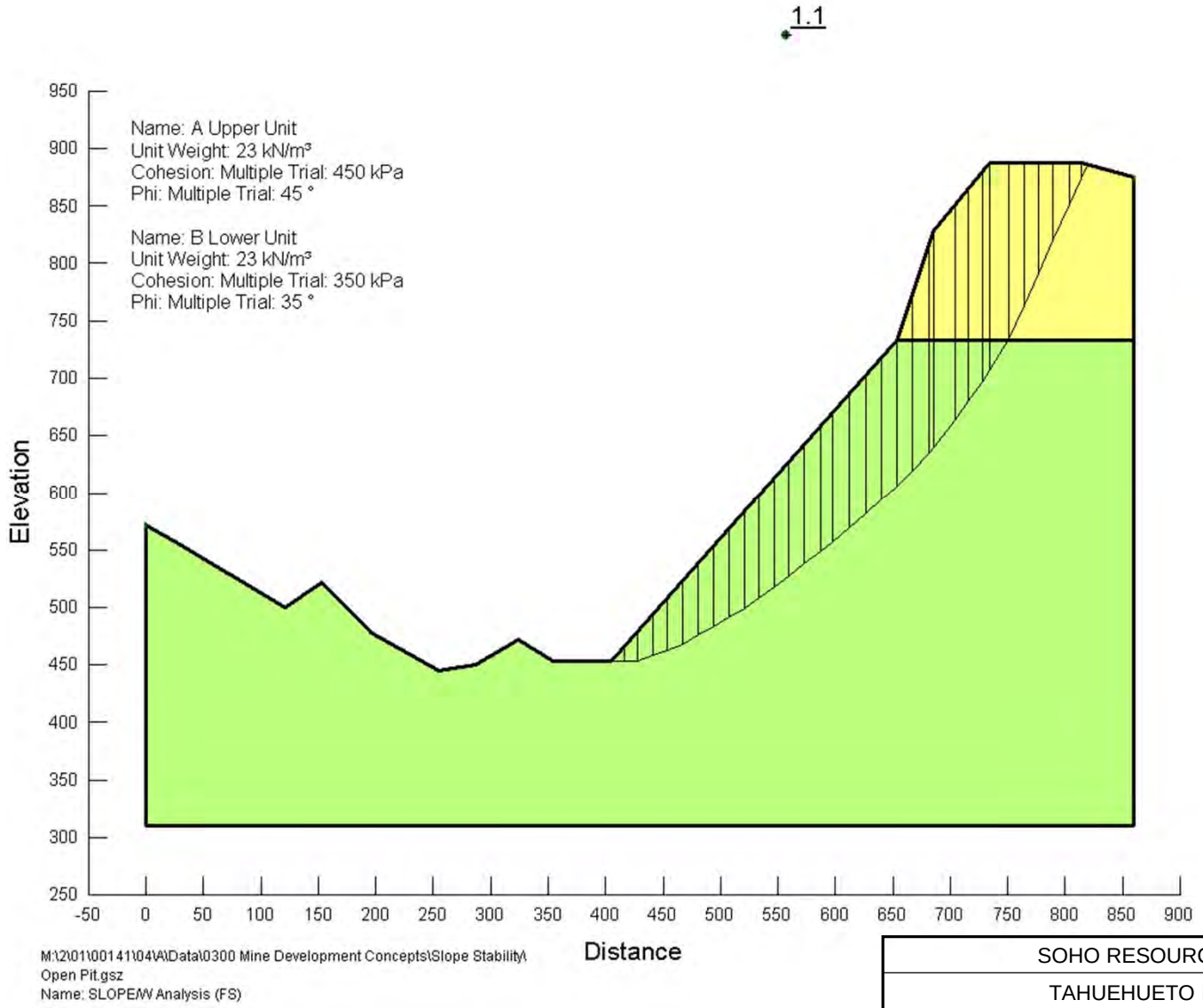
Knight Piésold CONSULTING

P/A NO. VA201-141/4

REF. NO. VA11-00817

FIGURE 9

REV 0



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Name: SLOPE/W Analysis (FS)

REV	DATE	DESCRIPTION	PREP'D	CHK'D	APP'D
0	08AUG'11	ISSUED WITH LETTER	LM	GIJ	KJB

SOHO RESOURCES CORP.

TAHUEHUETO PROJECT

CINCO DE MAYO
ROCK MASS STABILITY ANALYSES
OPEN PIT WEST WALL

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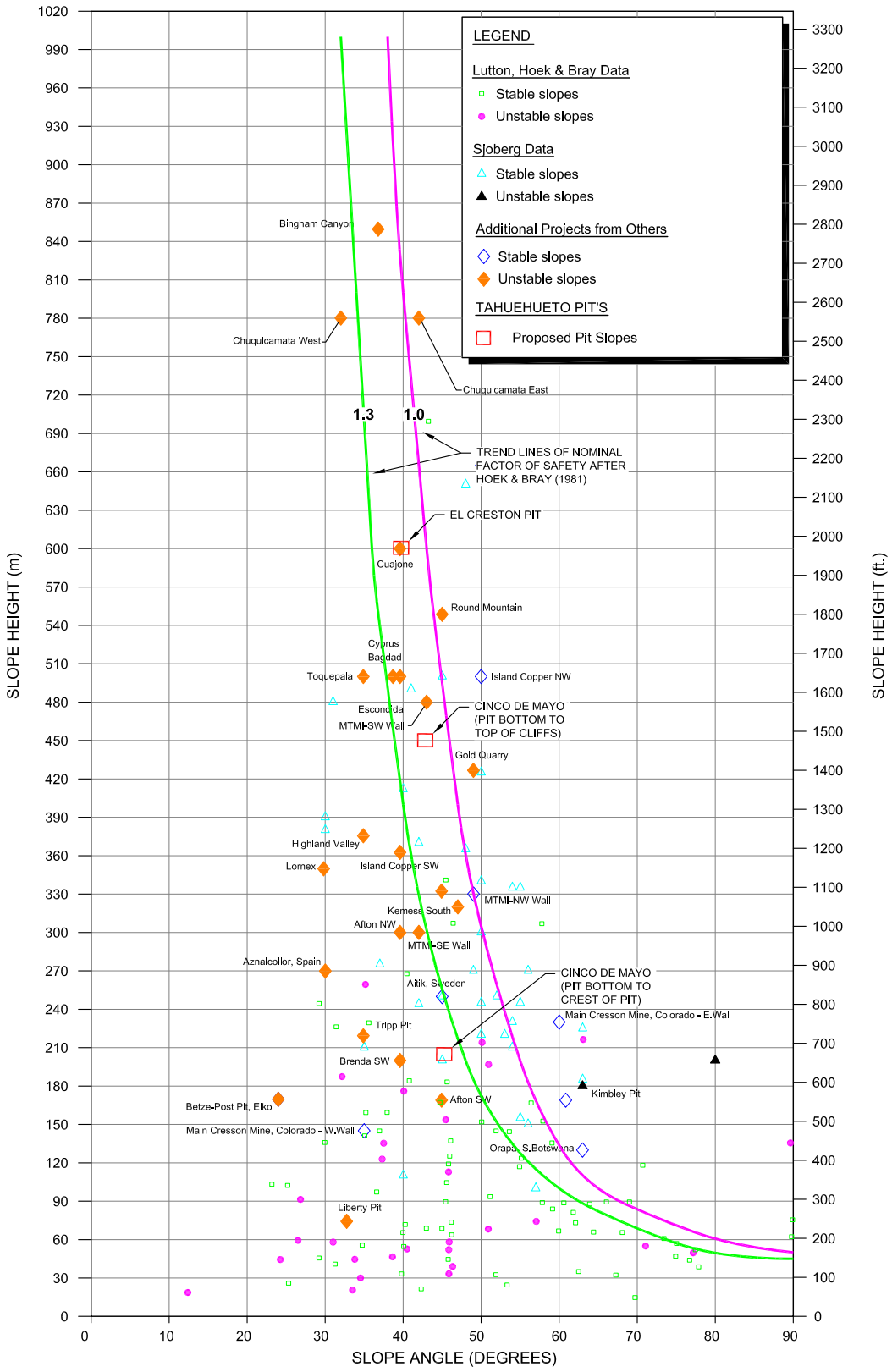
P/A NO. VA201-141/4

REF. NO. VA11-00817

FIGURE 10

REV 0



NOTES:

1. ORIGINAL DATA POINTS AFTER LUTTON 1970, HOEK AND BRAY 1981, AND SJOBERG 1996
2. ADDITIONAL DATA FROM KNIGHT PROJECTS AND OTHERS.

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SOHO RESOURCES CORP.

TAHUEHUETO PROJECT

SLOPE HEIGHT VERSUS SLOPE ANGLE PRECEDENT FOR HARD ROCK SLOPES

Knight Piésold CONSULTING

P/A NO. VA201-141/4

REF NO. VA11-00817

FIGURE 11

REV 0

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Knight Piésold
CONSULTING

APPENDIX A

SITE VISIT PHOTOGRAPHS

(Pages A-1 to A-9)



PHOTO 1 – Historic mill/plant site



PHOTO 2 – Conglomerate overburden at the historic mill/plant site.

SOHO RESOURCES CORP.
TAHUEHUETO PROJECT

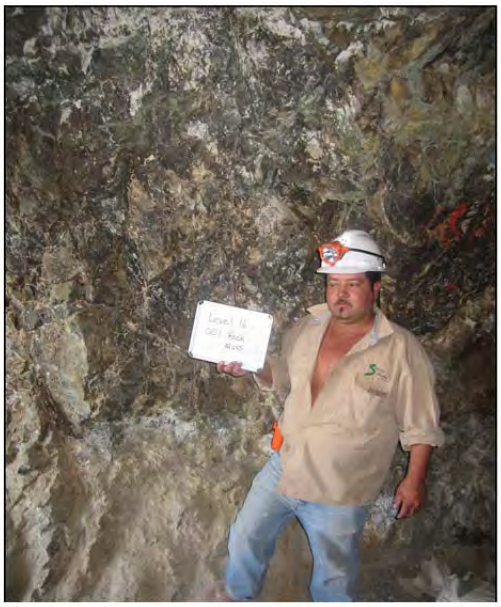

PHOTO 3 – El Creston Historic Underground Workings Level 16 (Entrance)



PHOTO 4 – El Creston Historic Underground Workings Level 14 (Historic explosives magazine)

SOHO RESOURCES CORP.
TAHUEHUETO PROJECT



PHOTO 5 – El Creston Historic Underground Workings Level 10 Drift (drift following the fault breccia)



PHOTO 6 – El Creston Historic Underground Workings Level 10 Cross Cut (cross cutting between fault breccia)



PHOTO 7 – El Creston Historic Underground Workings Level 10 Ore Pass (upper levels not visited)



PHOTO 8 – El Rey Historic Underground Workings Level 4 (note extensive white calcite deposits on the floor and walls)



PHOTO 9 – El Rey Historic Underground Workings Level 2 Portal (water ~ 1m deep)



PHOTO 10 – Cinco de Mayo Historic Underground Workings Level 1 Portal



PHOTO 11 – El Creston Pit Area (exploration camp lower left)



PHOTO 12 – Base of the cliffs above the El Creston deposit

SOHO RESOURCES CORP.
TAHUEHUETO PROJECT



PHOTO 13 – Cinco de Mayo – Potential Open Pit Outline

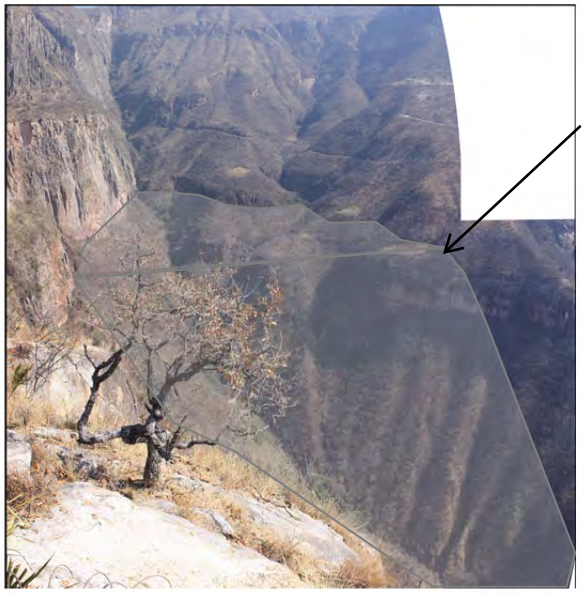

PHOTO 14 – Combined Filtered Tailings and Waste Rock Dump General Area

SOHO RESOURCES CORP.
TAHUEHUETO PROJECT



PHOTO 15 – Project Weather Station



PHOTO 16 – El Creston Level 16 Weir (Typical of the weirs constructed on site)



PHOTO 17 – Rio de las Vueltas (observed flow approximate flow 125 l/s)



PHOTO 17 – Rio de las Vueltas (observed flow approximate flow 125 l/s)